

ADDENDUM NO. 2

TRACK & FIELD IMPROVEMENTS - INC 1

LITTLE JOB #612-12353-01 DSA: A# 03-120008 File #56-H4

Adolfo Camarillo High School

4660 Mission Oaks Blvd. Camarillo, CA 93012

November 4, 2019

LITTLE

1300 Dove Street, Suite 100 Newport Beach, CA 92660

Architect- Jay R. Tittle, C- 12955

1. PART 1 - GENERAL

- 1.1. The following revisions and/or clarifications shall be made to the Bidding Requirements and Contract Documents. Revise and amend the Documents for the above-named project in accordance with this Addendum. The bid shall reflect these addendum changes and each bidder shall make reference in their bid to this addendum
- **1.2.** All Bidding Requirements and Contract Documents shall apply to this addendum as originally indicated in the applicable portions of the contract documents, unless otherwise modified by this addendum.
 - **1.2.1.** Addendum No. 1 issued on October 21, 2019

1.3. GENERAL CLARIFICATIONS

- **1.3.1.** The following clarifications are issued to all bidders as information for use in preparing bids:
 - **1.3.1.1.** The job walk is not mandatory.
 - **1.3.1.2.** The prime contractor may hire a general or subcontractor with the CMAS base preparation experience requirements to satisfy the CMAS base preparation requirements.
 - **1.3.1.3.** The estimate for bonding purposes of this project is \$2,650,000.
 - **1.3.1.4.** The duration for the construction of this project is 182 calendar days.

2. PART 2 - PROJECT MANUAL

2.1. NONE ISSUED

3. PART 3 - DRAWINGS

3.1. CIVIL DRAWINGS ISSUED

- **3.1.1.** The following Addendum ("AD") Drawings, marked Delta 2, are issued:
 - **3.1.1.1.** Drawing C-1.1/AD1-C1: Replace with Drawing AD2-C1.
 - **3.1.1.2.** Drawing C-2.0: Replace with Drawing AD2-C2.
 - **3.1.1.3.** Drawing C-3.0: Replace with Drawing AD2-C3.
 - **3.1.1.4.** Drawing C-3.1: Replace with Drawing AD2-C4.
 - **3.1.1.5.** Drawing C-3.2: Replace with Drawing AD2-C5.
 - **3.1.1.6.** Drawing C-3.3: Replace with Drawing AD2-C6.
 - **3.1.1.7.** Drawing C-4.0/AD1-2: Replace with Drawing AD2-C7.
 - **3.1.1.8.** Drawing C-4.1: Replace with Drawing AD2-C8.
 - **3.1.1.9.** Drawing C-4.2: Replace with Drawing AD2-C9.
 - **3.1.1.10.** Drawing C-4.3: Replace with Drawing AD2-C10.
 - **3.1.1.11.** Drawing C-5.0/AD1-C3: Replace with Drawing AD2-C11.
 - **3.1.1.12.** Drawing C-5.1/AD1-C4: Revise currently issued Drawing per AD2-C12.
 - 3.1.1.13. Drawing C-5.2/AD1-C5: Replace with Drawing AD2-C13.
 - **3.1.1.14.** Drawing C-5.3/AD1-C6: Replace with Drawing AD2-C14.

3.2. ARCHITECTURAL DRAWINGS ISSUED

- **3.2.1.** The following Addendum ("AD") Drawing, marked Delta 2, is issued:
 - **3.2.1.1.** Drawing A1.0.1: Revise currently issued Drawing per AD2-A1.

3.3. ELECTRICAL DRAWINGS ISSUED

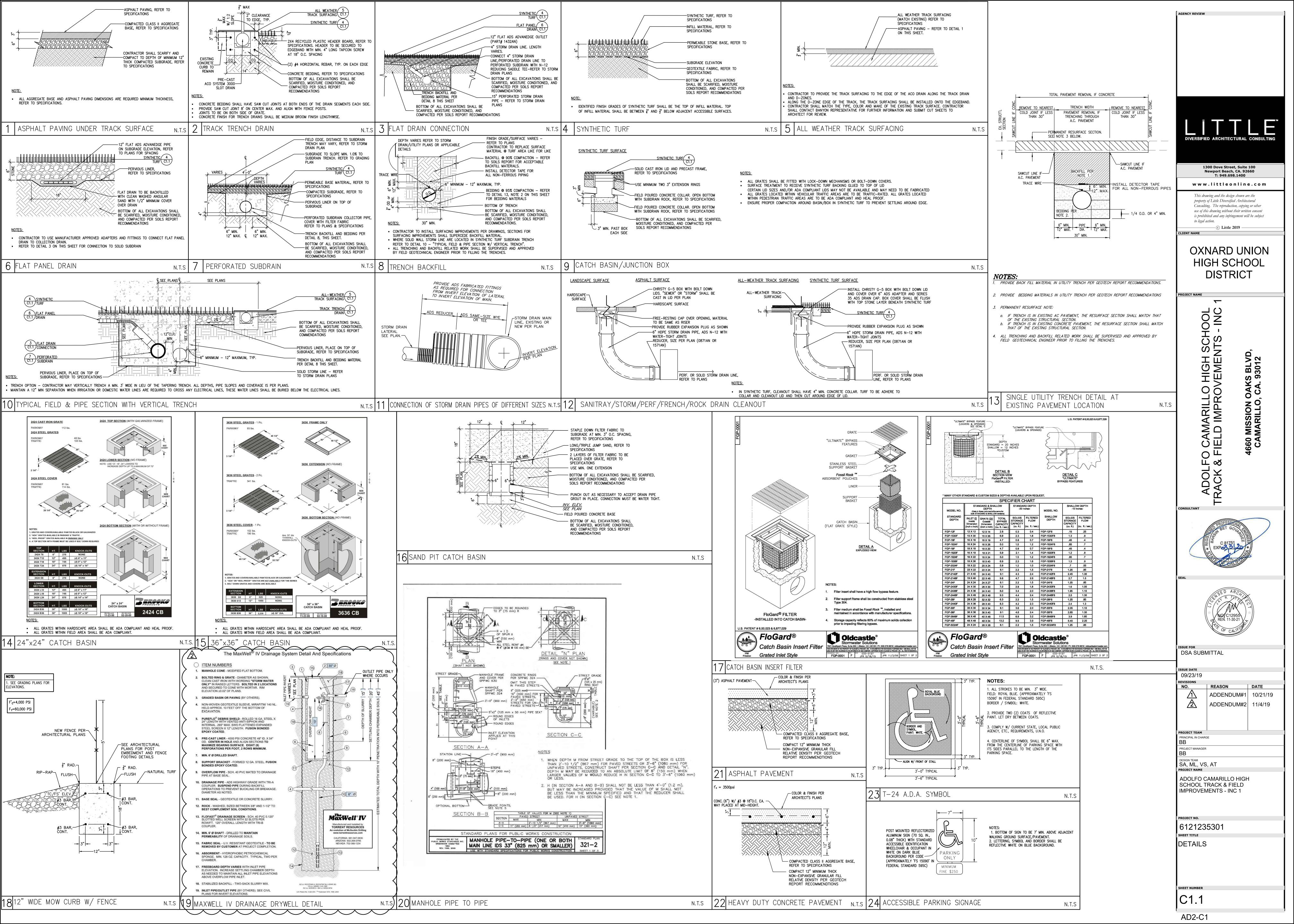
- **3.3.1.** The following Addendum ("AD") Drawings, marked Delta 2, are issued:
 - **3.3.1.1.** Drawing E-000: Replace with Drawing AD2-E1.
 - **3.3.1.2.** Drawing E-100: Replace with Drawing AD2-E2.

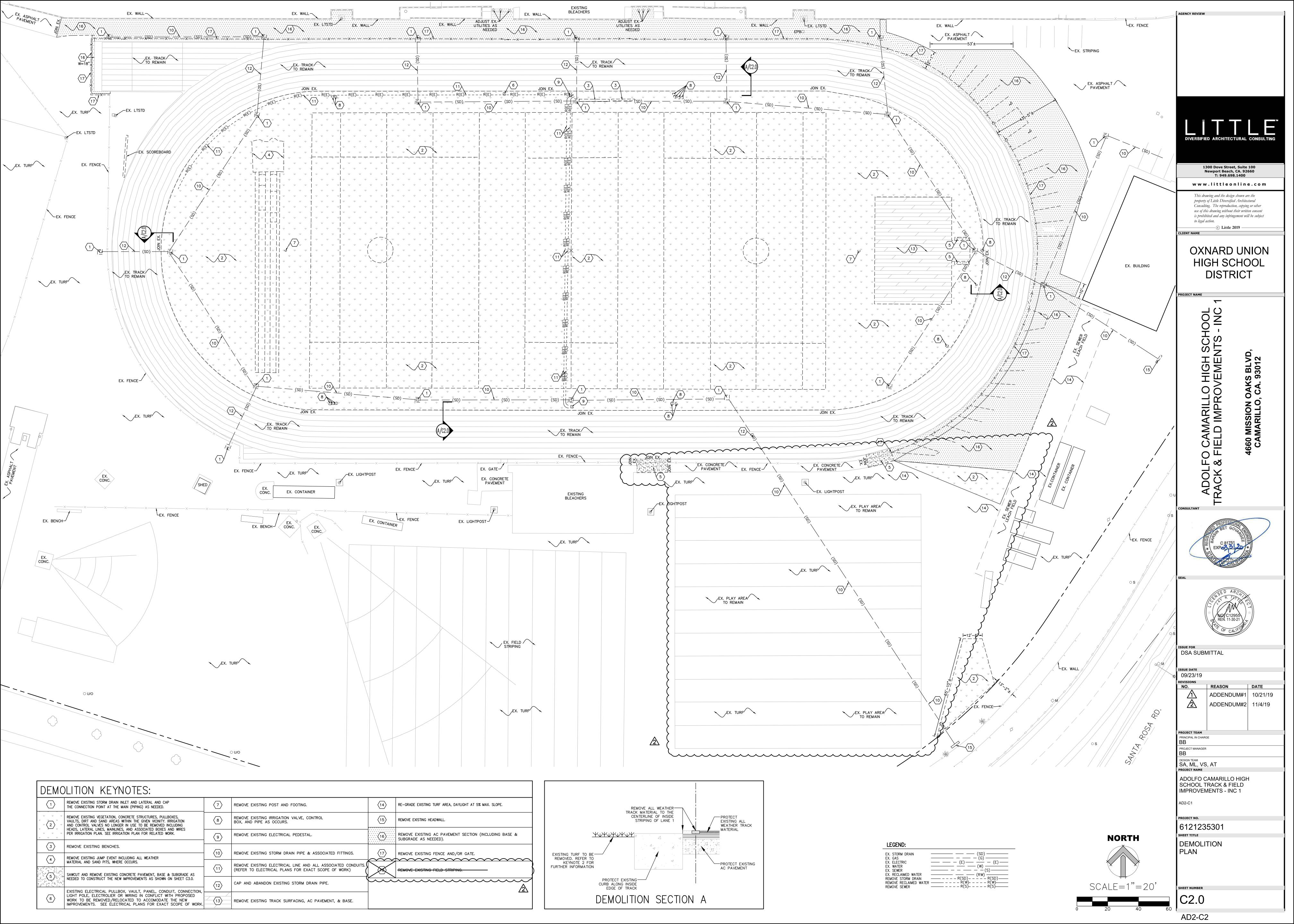
- **3.3.1.3.** Drawing E-200: Replace with Drawing AD2-E3.
- **3.3.1.4.** Add new Drawing AD2-E4, For Reference Only
- **3.3.1.5.** Add new Drawing AD2-E5, For Reference Only
- **3.3.2.** Narrative clarifications to Electrical Drawings are issued as follows:
 - **3.3.2.1.** Drawing E-200 Clarify as follows:
 - **3.3.2.1.1.** Existing panel locations at existing switchboard are shown on E-100 (AD2-E2) and referenced on 'as-built plans' AD2-E4 & AD2-E5.
 - **3.3.2.1.2.** For box sizes refer to sheet AD2-E1 and AD2-E3 for quantity & size of conduit connected to each junction box, to determine box size at each location. Contractor to refer to 'as-built plans' AD2-E4 & AD2-E5 for box sizes, including P1, P2, P4 & P5.
 - **3.3.2.1.3.** Contractor to refer to note P7/E200 (AD2-E3) to clarify removal of Stadium Light Fixtures and speakers, cap wire at existing connections and deliver fixtures and speakers to District.
 - **3.3.2.1.4.** Junction boxes to be concrete type. Contractor shall field verify for existing junction boxes to be replaced with the same size, as per 'as-built plans' AD2-E4 & AD2-E5.

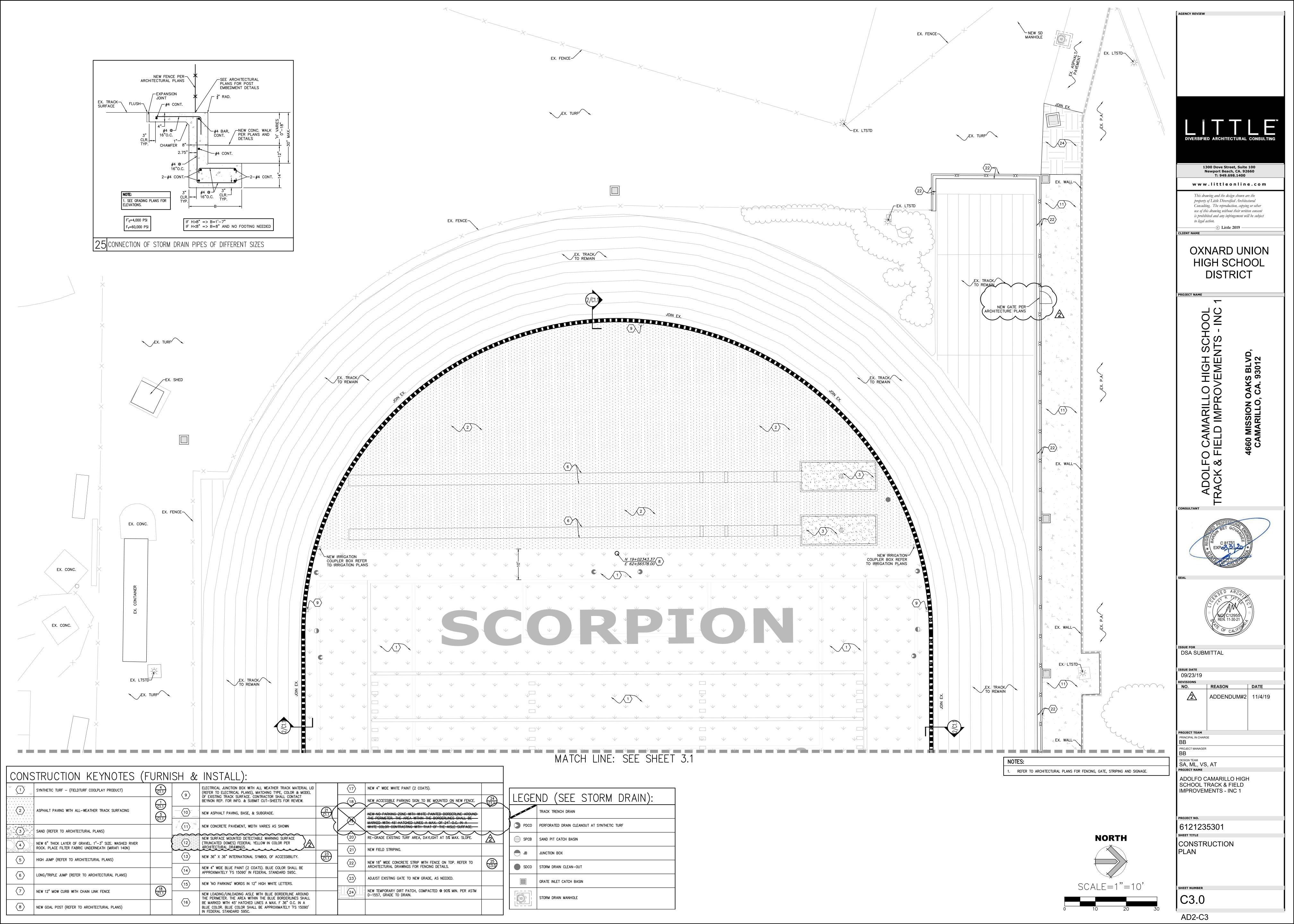
END OF ADDENDUM NO. 2

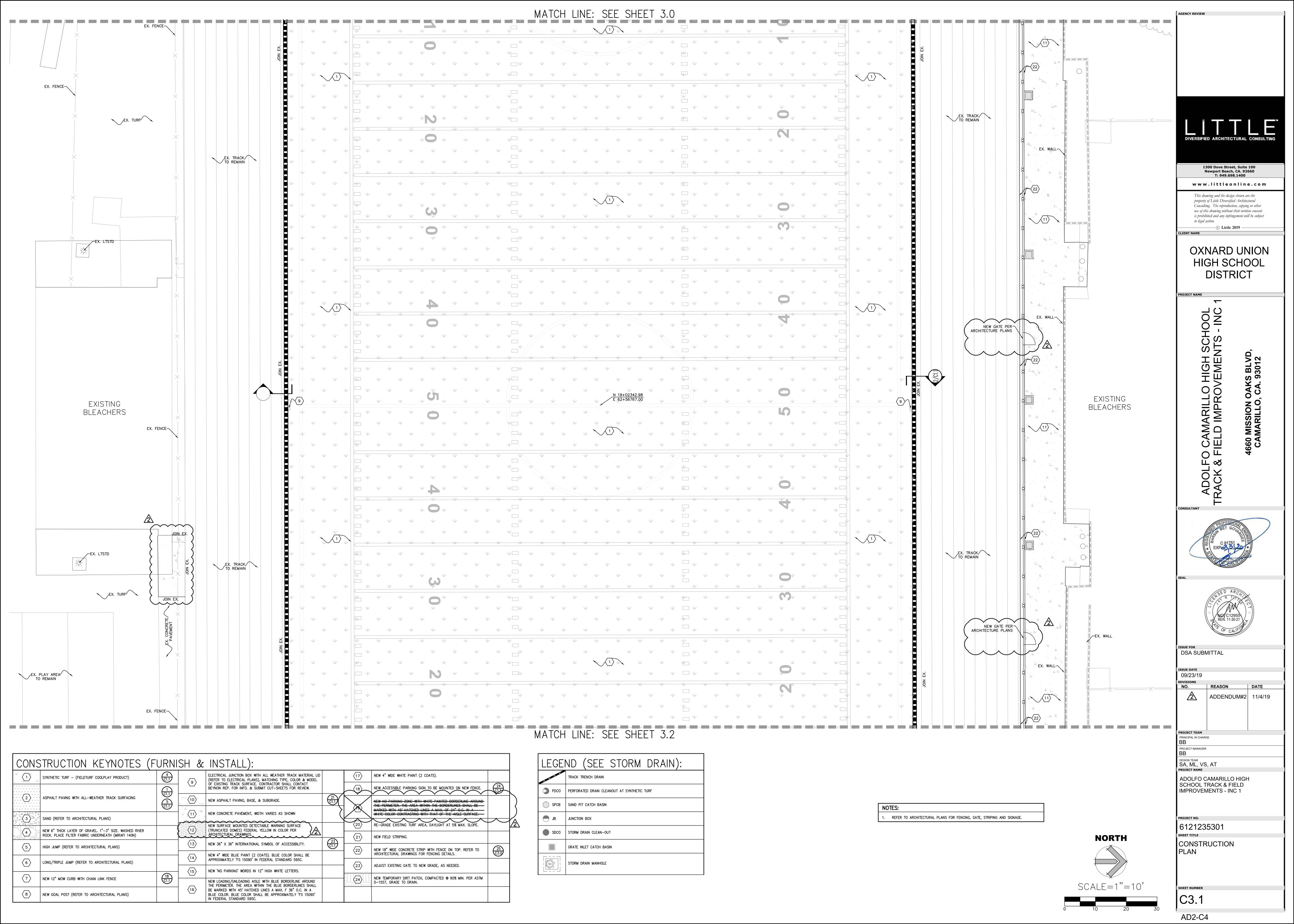
Enclosures:

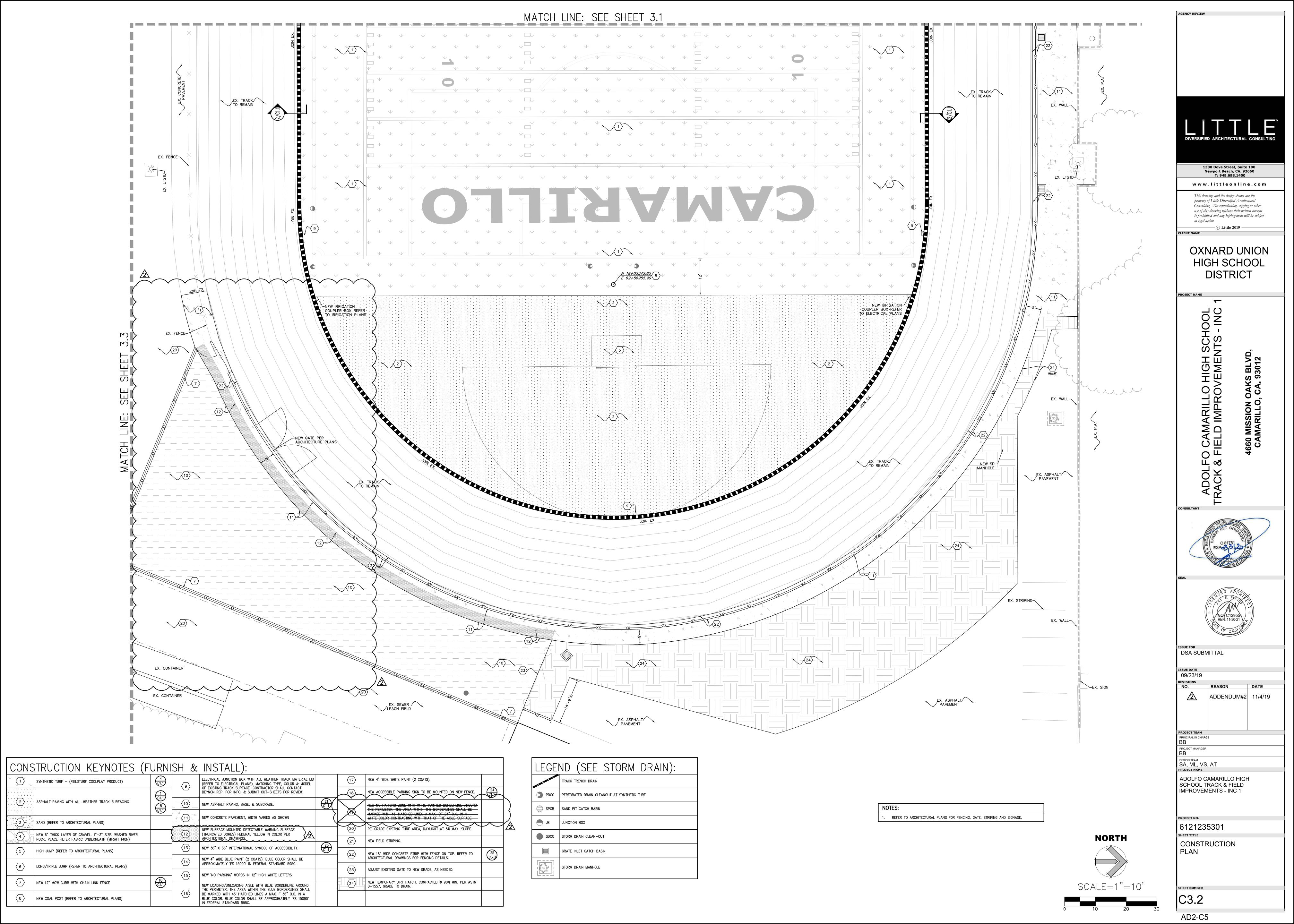
- I) New Project Manual Documents Issued:
 - a) None
- II) New full-size Drawings Issued:
 - a) Drawings AD2-C1 through AD2-C14, Delta 2.
 - b) Drawings AD2-A 1, Delta 2.
 - c) Drawings AD2-E1 through AD2-E5, Delta 2.











EXCERPTS FROM GEOTECHNICAL REPORT

Report No.: 19-8-3 (Revised) Samples of near-surface soils were tested for pH, resistivity, soluble sulfates, and soluble | Proposed areas of athletic field improvements or areas to receive fill should be overexcavated to chlorides. The test results provided in Appendix B should be distributed to the design team for a depth of one foot. The resulting surface should then be scarified an additional 6 inches, their interpretations pertaining to the corrosivity or reactivity of various construction materials | moisture conditioned, and recompacted. This will result in at least 12 inches of compacted fill (such as concrete and piping) with the soils. It should be noted that sulfate contents (61 mg/Kg) | below the flat panel drains, and 18 inches of compacted fill below the areas between the drains. are in the "S0" ("negligible") exposure class of Table 19.3.1.1 of ACI 318-14; therefore, it appears | Compaction should be verified to be a minimum of 90% of the maximum dry density obtained by that special concrete designs will not be necessary for the measured sulfate contents.

Based on criteria established by the County of Los Angeles (2013), measurements of resistivity of Proposed areas of track surface replacements (and underlying asphaltic concrete pavement), near-surface soils (6,000 ohms-cm) indicate that they are "moderately corrosive" to ferrous metal exterior slabs-on-grade, or sidewalks should be overexcavated to a depth of one foot. The (i.e. cast iron, etc.) pipes.

GEOTECHNICAL CONCLUSIONS

implemented into the project.

Infiltration of storm water may be feasible for this campus. More detailed findings after infiltration testing is completed.

GEOTECHNICAL RECOMMENDATIONS FOR FIELD AND TRACK SURFACE IMPROVEMENTS

All proposed grading should conform to the 2016 California Building Code.

include the grading plans, drainage plans, and applicable details.

debris should be stockpiled away from areas to be graded, and ultimately removed from the site included in these figures. to prevent their inclusion in fills. Voids created by removal of such material should be properly observed by the Geotechnical Engineer.

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elevation of each field.

It is recommended that Earth Systems be retained to provide Geotechnical Engineering services during the site development, drain installation, and grading phases of the work to observe Overexcavation and recompaction of soils under and around pier footings for the entry gates will construction.

GRADING RECOMMENDATIONS FOR BUILDINGS, ENTRY GATES, AND PAVEMENTS

the fault zone, a conventional foundation system would be acceptable.

Grading at a minimum should conform to the 2016 California Building Code.

The existing ground surface should be initially prepared for grading by removing all vegetation trees, large roots, debris, other organic material, and non-complying fill. Non-complying fill On-site soils may be used for fill once they are cleaned of all organic material, rock, debris, and would include the gravel and piping of the leach lines that reportedly exist southeast of the irreducible material larger than 8 inches. eastern end of the track around the perimeter of the football field. Organics and debris should be stockpiled away from areas to be graded, and ultimately removed from the site to prevent | Fill and backfill should be placed at, or slightly above optimum moisture in layers with loose by the Geotechnical Engineer.

Once the gravel and piping is completely removed from the existing leach lines, the excavations should be deepened and widened until firm native soils are encountered in each direction.

Report No.: 19-8-3 (Revised) prequalified by the Geotechnical Engineer. Final comments on the characteristics of the import

will be given after the material is at the project site.

August 28, 2019

stabilization of the excavation bottom will be required prior to placing fill. This can be provided in the California Building Code for clay soils. accomplished by various means. The first method would include drying the soils as much as

possible through scarification, and working thin lifts of "6-inch minus" crushed angular rock into the excavation bottom with small equipment (such as a D-4) until stabilization is achieved. Use rock prior to placement of filter fabric (until the bottom is stabilized). The rock should then be Miscellaneous Base (PMB) compacted to a minimum of 95% of the maximum dry density on reinforcement can be added to the pavement section at approximately two inches below the top covered with a geotextile filter fabric before placing fill above. It is anticipated that stabilization subgrade soils compacted to a minimum of 95% of the maximum dry density. will probably be necessary due to the existing high moistures of the soils, and due to the shallow

compacted to 90% of the maximum dry density. Backfill of offsite service lines will be subject to maximum dry density. the specifications of the approved project plans or this report, whichever are greater.

compliance with these recommendations.

Utility trenches running parallel to footings should be located at least 5 feet outside the footing line, or above a 2:1 (horizontal to vertical) projection downward from a point 9 inches above the outside edge of the bottom of the footing.

Compacted native soils should be utilized for backfill below structures. Sand should not be used under structures because it provides a conduit for water to migrate under foundations.

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the ASTM D 1557 test method.

resulting surface should then be scarified an additional 6 inches, moisture conditioned, and ecompacted. Compaction should be verified to be a minimum of 95% of the maximum dry density obtained by the ASTM D 1557 test method.

The site is suitable for the proposed athletic field improvements from a Geotechnical Engineering Once subgrade elevations are achieved and flat panel drains are installed, a permeable filter standpoint provided that the recommendations contained in this report are successfully fabric, such as Mirafi 140N, should be placed over the subgrade soils and panel drains. Permeable base should be placed over the filter fabric and compacted to a minimum of 95% of the maximum dry density obtained by the ASTM D 1557 test method.

> The bottoms of all excavations should be observed by a representative of this firm prior to rocessing or placing fill.

On-site soils may be used for fill once they are cleaned of all organic material, rock, debris, and irreducible material larger than 8 inches.

Plans and specifications should be provided to Earth Systems prior to grading. Plans should | Fill and backfill should be placed at, or slightly above optimum moisture in layers with loose thickness not greater than 8 inches.

The existing ground surface should be initially prepared for grading by removing all grass and Shrinkage of soils affected by compaction is estimated to be about 10% based on an anticipated vegetation, large roots, debris, other organic material, and non-complying fill. Organics and average compaction of 92%. Shrinkage from removal of any existing subsurface structures is not

backfilled and compacted. No compacted fill should be placed unless the underlying soil has been Utility trench backfill should be governed by the provisions of this report relating to minimum compaction standards. In general, on-site service lines may be backfilled with native soils compacted to 90% of the maximum dry density. Backfill of offsite service lines will be subject to the specifications of the jurisdictional agency or this report, whichever are greater.

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Overexcavation and recompaction of soils in the building areas will be necessary to decrease the Compaction tests shall be made to determine the relative compaction of the fills, subgrade soils, potential for differential settlement and provide more uniform bearing conditions. Soils should and utility trench backfills in accordance with the following minimum guidelines: one test for each | be overexcavated to a depth of 4.5 feet below finished subgrade elevation throughout the entire two-foot vertical lift, one test for each 1,000 cubic yards of material placed, one test per two-foot | building area, and to a distance of 5 feet beyond the perimeter of each building. The resulting vertical lift per 250 lineal feet of utility trench backfill, and four tests at finished subgrade surface should then be scarified an additional 6 inches, moisture conditioned, and recompacted to at least 90% of the maximum dry density. The intent of these recommendations is to have a minimum of 5 feet of compacted soil below the building.

compliance with the design concepts, specifications and recommendations, and to allow design | also be necessary. Soils should be overexcavated to a depth of 4.5 feet below finished subgrade changes in the event that subsurface conditions differ from those anticipated prior to the start of elevation, and to a distance of 3 feet on either side of the footing edges. The resulting surface should then be scarified an additional 6 inches, moisture conditioned, and recompacted to at least 90% of the maximum dry density.

Areas outside of the building area to receive fill, exterior slabs-on-grade, sidewalks, or paving t should be noted that the location provided to Earth Systems for the future 498 square-foot | should be overexcavated to a depth of 1.5 feet below finished subgrade elevation. The resulting restroom building is within the Fault Rupture Hazard Zone for the Camarillo Fault, and an surface should then be scarified an additional 6 inches, moisture conditioned, and recompacted. evaluation of the fault rupture hazard may be required. However, if the size precludes the Because the expansion index of on-site soils is in the "very low" range, no aggregate base will be requirement for hazard evaluation, or an acceptable location for the restroom is located outside required below sidewalks. (Recommendations for structural paving sections for pavements subjected to vehicular traffic are provided elsewhere in this report.)

> The bottoms of all excavations should be observed by a representative of this firm prior to processing or placing fill.

their inclusion in fills. Voids created by removal of such material should be properly backfilled | thickness not greater than 8 inches. Each layer should be compacted to a minimum of 90% of the and compacted. No compacted fill should be placed unless the underlying soil has been observed | maximum dry density obtainable by the ASTM D 1557 test method. The upper one foot of subgrade below areas to be paved should be compacted to a minimum of 95% of the maximum

> mport soils used to raise site grade should be equal to, or better than, on-site soils in strength, xpansion, and compressibility characteristics. Import soil can be evaluated, but will not be Project No.: 303275-001 August 28, 2019

DESIGN VALUES FOR FENCEPOST PIER FOOTINGS IN NON-COMPACTED AREAS

Pier footings to support fence posts that are drilled into native soils may be designed for passive f pumping soils or otherwise unstable soils are encountered during the overexcavation, pressures of 100 psf per foot below natural grade. This value is based on presumptive parameters

PRELIMINARY ASPHALT PAVING SECTIONS FOR TRACK RESURFACING

of a geotextile fabric such as Mirafi 500X, or Tensar TX-160, or an approved equivalent, is another Assuming a Traffic Index of 5 for areas to be used for asphalt below track resurfacing, and using possible means of stabilizing the bottom. If this material is used, it should be laid on the the measured R-Value of 29, paving sections should have a minimum gravel equivalent or excavation bottom and covered with approximately 12 inches of "3-inch minus" crushed angular | 1.14 feet. This can be achieved by using 3 inches of Processed | If additional resistance to cracking is desired beyond that provided by the contraction joints, steel

groundwater depth. Unit prices should be obtained from the Contractor in advance for this work. For new fire lanes or drive lanes in parking areas with a Traffic Index of 6.5, paving sections should have a minimum gravel equivalent of 1.48 feet. This can be achieved by using 4 inches of Utility trench backfill should be governed by the provisions of this report relating to minimum asphaltic concrete on 9 inches of Processed Miscellaneous Base (PMB) compacted to a minimum On August 22, 2019, a set of two 8-inch diameter infiltration borings (P-1 and P-2) were drilled to compaction standards. In general, on-site service lines may be backfilled with native soils of 95% of the maximum dry density on subgrade soils compacted to a minimum of 95% of the depths of about 7 and 18 feet below the existing ground surface to determine the soil profile and

he preliminary paving sections provided above have been designed for the type of traffic Logs of Borings in Appendix A). Utility backfill operations should be observed and tested by the Geotechnical Engineer to monitor | indicated. If the pavement is placed before construction on the project is complete, construction loads, which could increase the Traffic Indices above those assumed above, should be taken into After drilling was completed, 3-inch diameter slotted PVC casings were lowered into the

PRELIMINARY CONCRETE PAVING SECTIONS

Concrete paving sections provided below have been based on an assumed design life of 20 years | taken at reasonable time intervals based on infiltrating rate, and after each of these intervals, and have been calculated for the measured R-Value of 29 (approximately equivalent to a water was added to return the water level to its original depth above the hole bottom for the coefficient of subgrade reaction of k = 150 pounds per cubic inch) using design methods next test interval. The tests were run until the infiltration rates were reasonably stable. presented by the American Concrete Institute (ACI 330R-87). For an assumed Traffic Index of 5 (for light traffic), the following minimum unreinforced paving section was determined:

- Concrete thickness =
- Aggregate base thickness under concrete = 3. Compressive strength of concrete, fc =

3,500 psi at 28 days

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530 psi

Maximum spacing of contraction joints, each way= 12.5 feet For an assumed Traffic Index of 6.5 (for traffic that includes fire trucks), the following minimum

4. Modulus of flexural strength of 3,500 psi concrete =

- unreinforced paving section was determined: Concrete thickness = 6 inches Aggregate base thickness under concrete = 4 inches
- Compressive strength of concrete, fc = 3,500 psi at 28 days 4. Modulus of flexural strength of 3,500 psi concrete = 530 psi Maximum spacing of contraction joints, each way= 15 feet

of concrete; however, reinforcement is not required.

STORM WATER INFILTRATION FEASIBILITY TESTING

allow installation of plastic casing for infiltration testing (see Site Plan in Appendix A for infiltration boring locations). All infiltration borings were bottomed into native Alluvium (see

boreholes. The annuli between the casings and boring walls were then filled with pea gravel. The falling-head borehole infiltration test procedure was used for infiltration testing. Approximately 2 feet of water was added to the bottom of each of the holes to start the tests, and the drop in the water surface monitored by taking periodic measurements. Readings were

t should be noted that the rate the water surface drops in a borehole is a percolation rate, which s related to, but is not an infiltration rate. Percolation rate ignores the wetted soil surface area into which the water is infiltrating and does not account for the volume of water infiltrated. An

NOTES:

REFER TO ARCHITECTURAL PLANS FOR FENCING, GATE, STRIPING AND SIGNAGE.

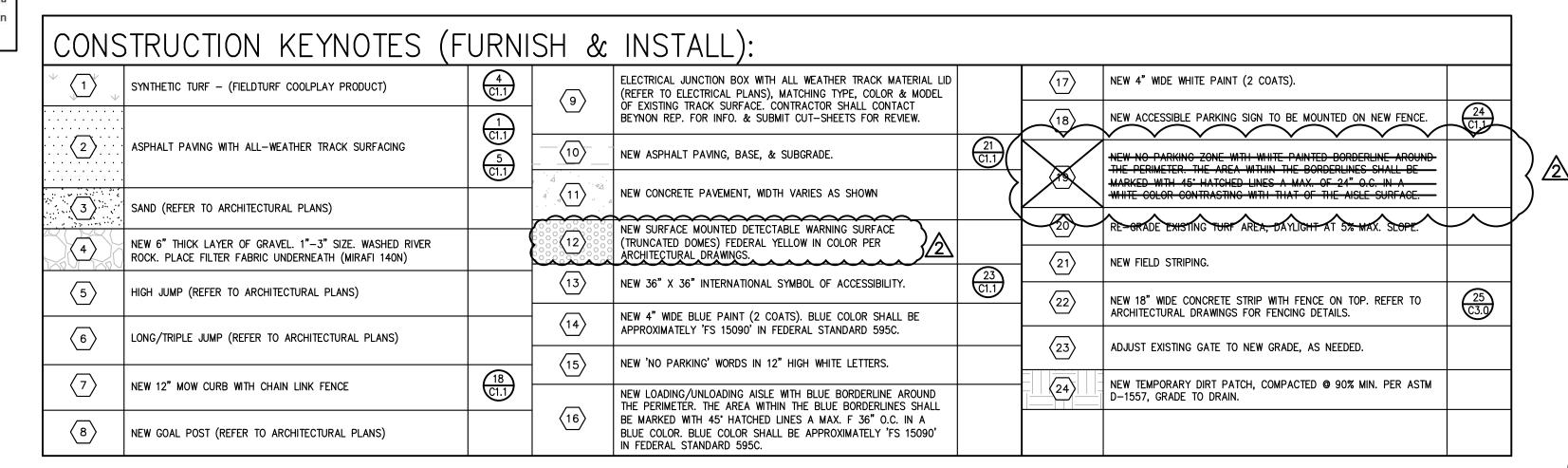
TRACK TRENCH DRAIN PDCO PERFORATED DRAIN CLEANOUT AT SYNTHETIC TURF SPCB | SAND PIT CATCH BASIN JUNCTION BOX SDCO | STORM DRAIN CLEAN-OUT

LEGEND (SEE STORM DRAIN):

GRATE INLET CATCH BASIN

STORM DRAIN MANHOLE

EX. TURF EX. PLAY AREA EX. PLAY AREA EX. SEWER / LEACH FIELD ∠EX. FENCE





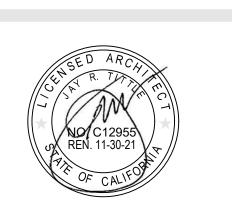
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OXNARD UNION HIGH SCHOOL





DSA SUBMITTAL

09/23/19

ADDENDUM#2 11/4/19

RINCIPAL IN CHARGE PROJECT MANAGER

SA, ML, VS, AT

ADOLFO CAMARILLO HIGH SCHOOL TRACK & FIELD IMPROVEMENTS - INC 1

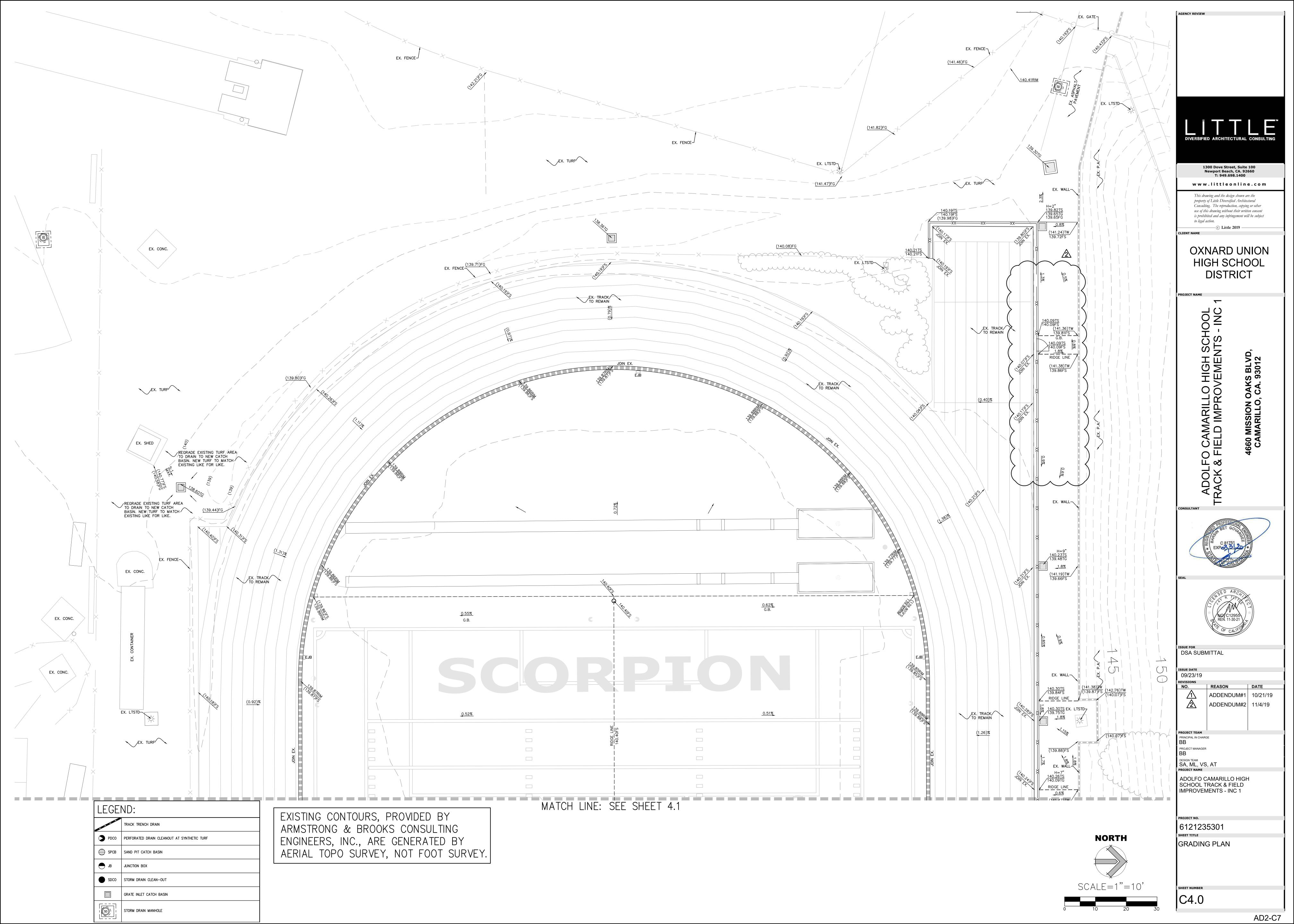
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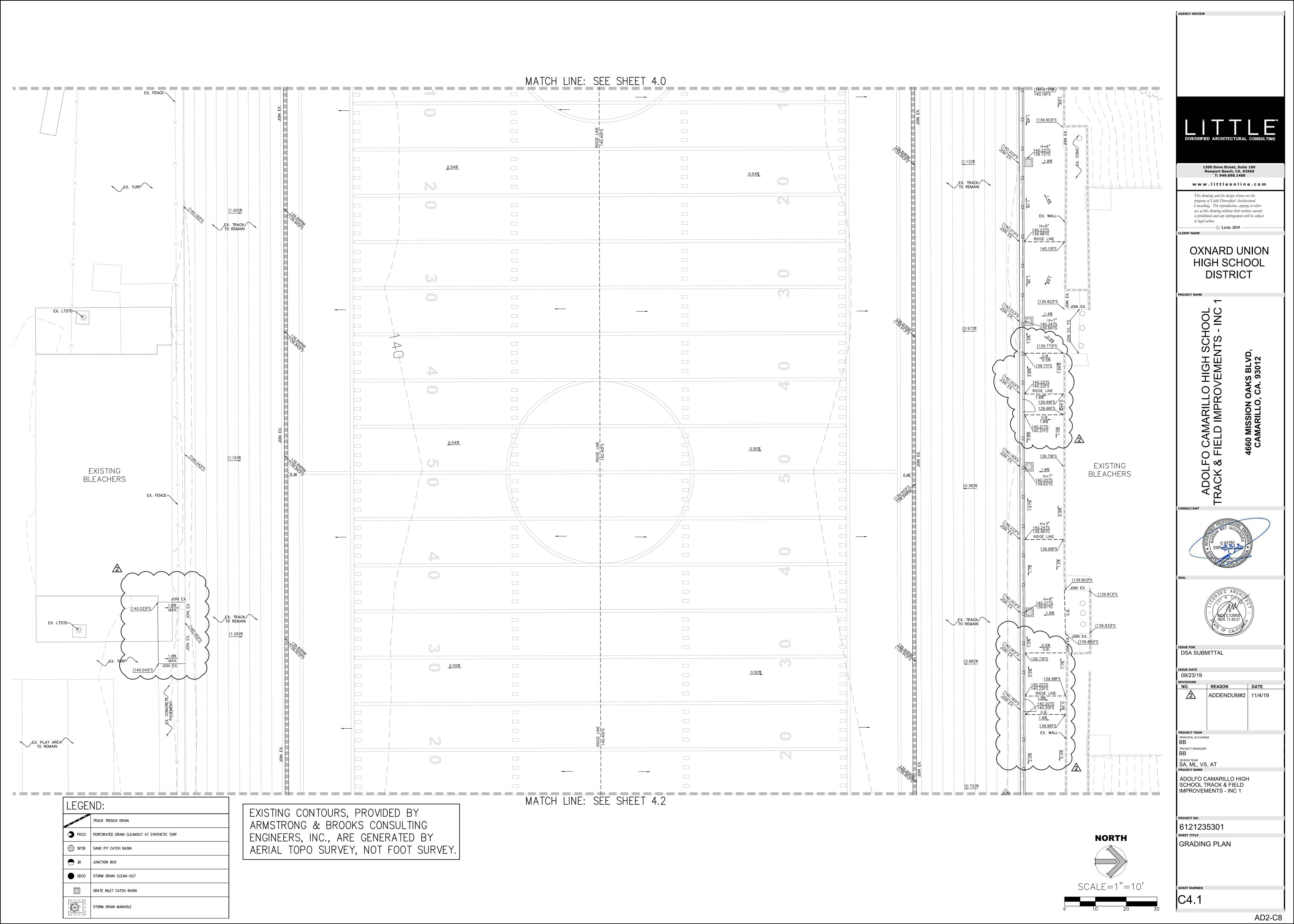
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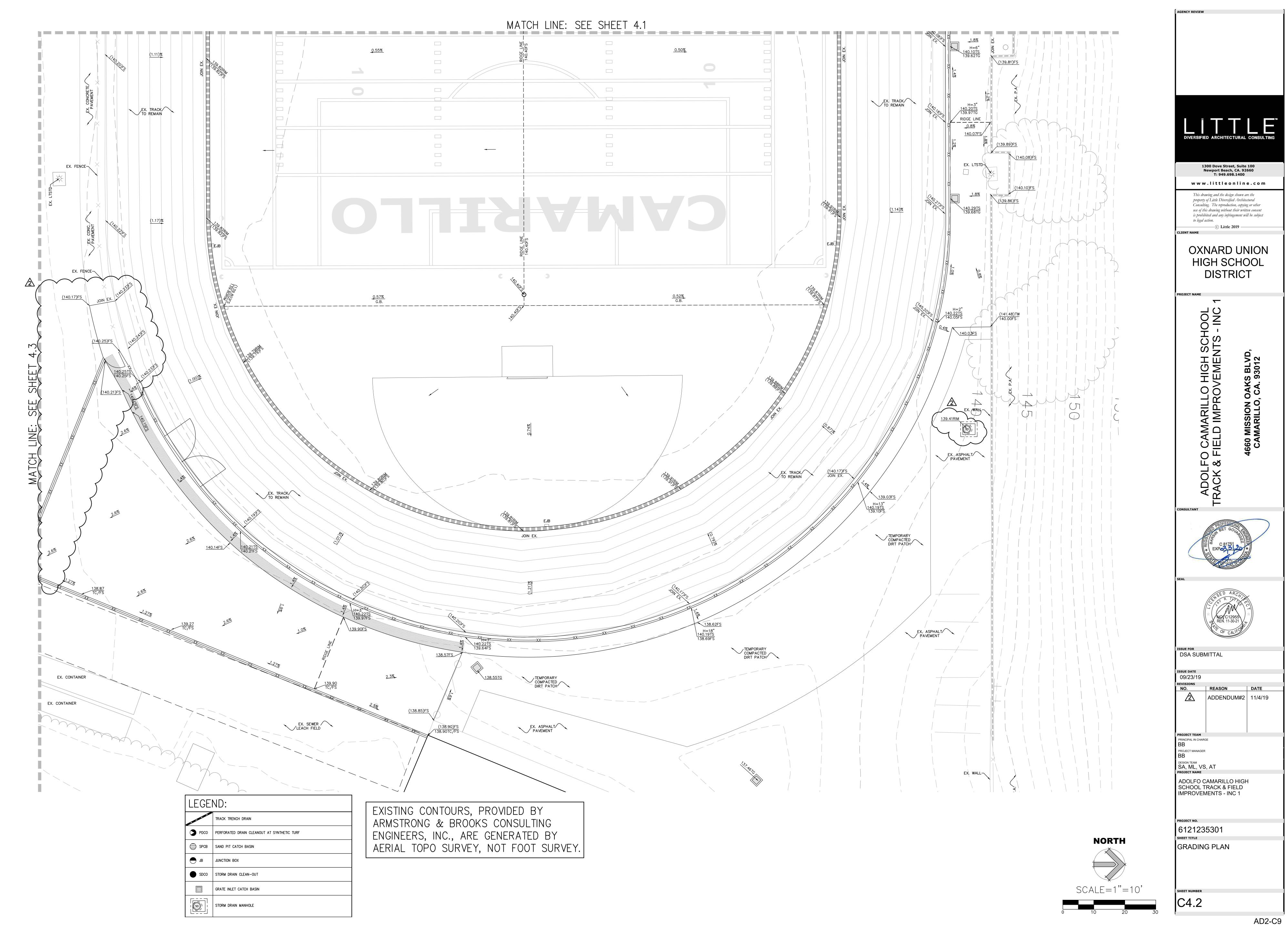
AD2-C6

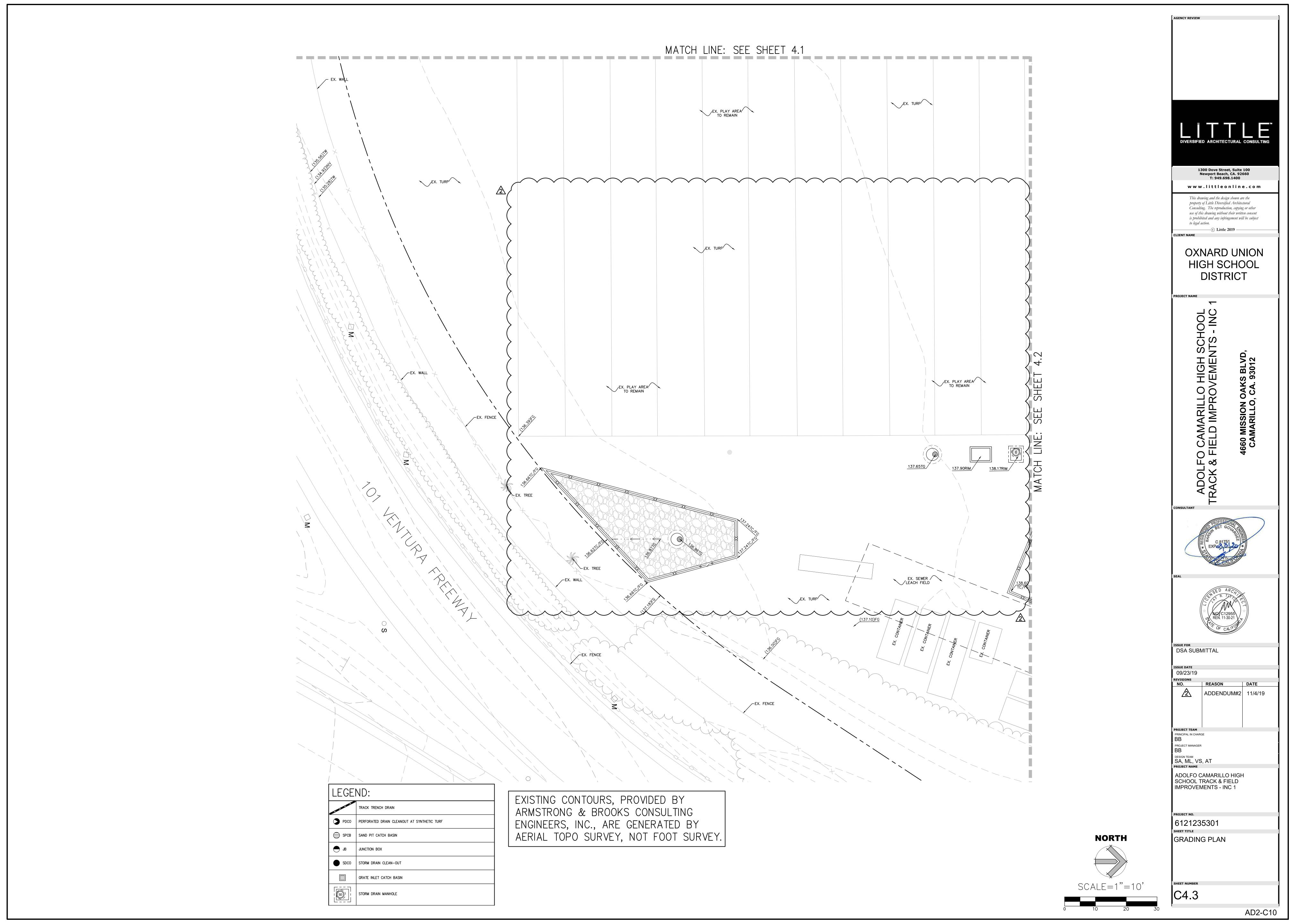
NORTH

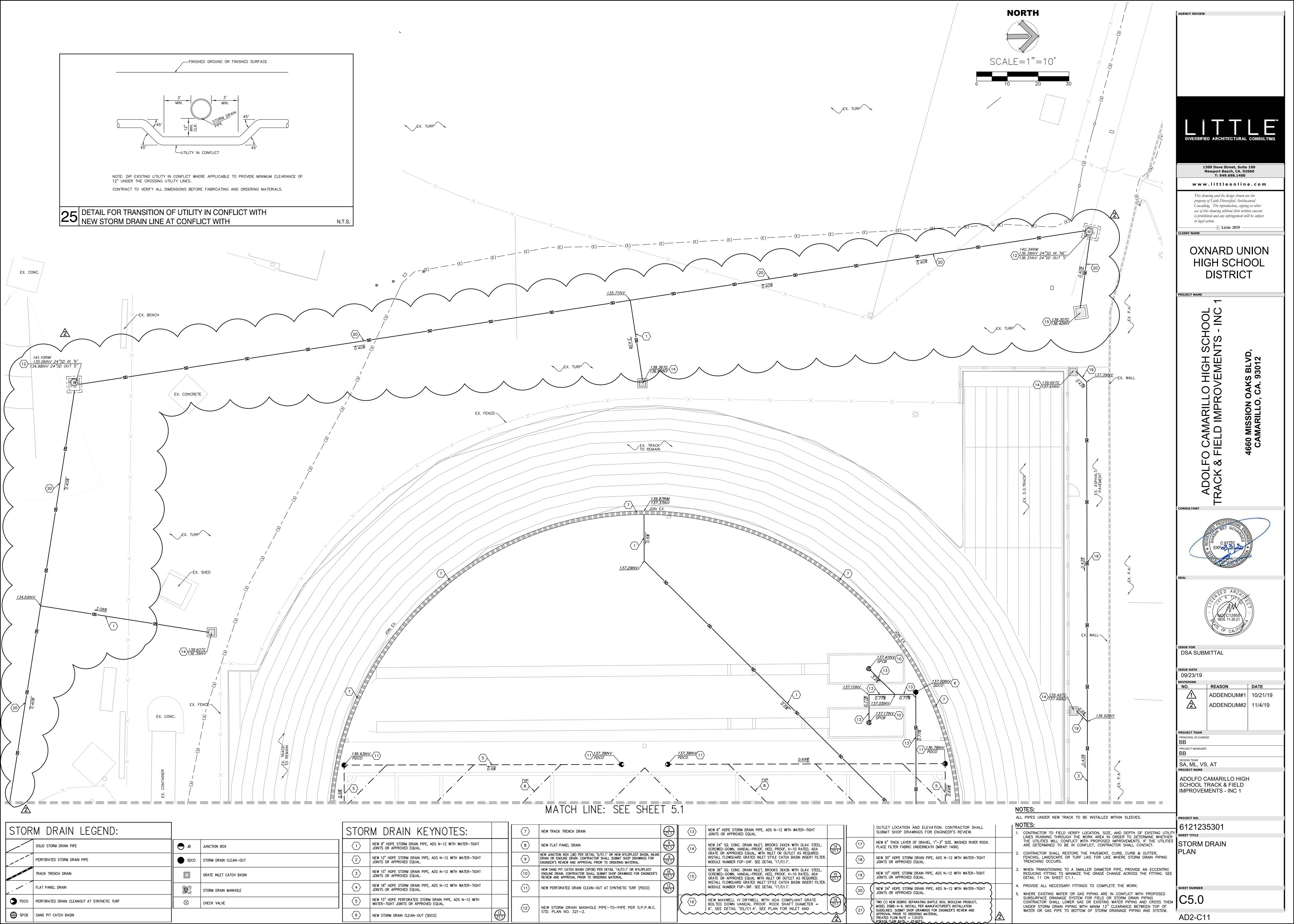
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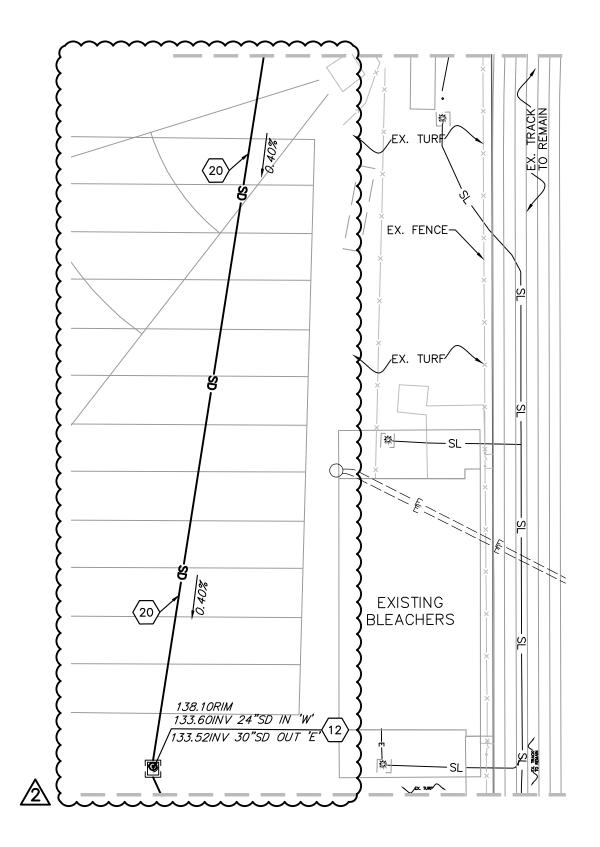


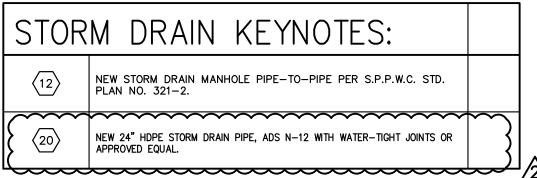








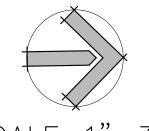








NORTH



SCALE=1"=30'



ADOLFO CAMARILLO HIGH SCHOOL

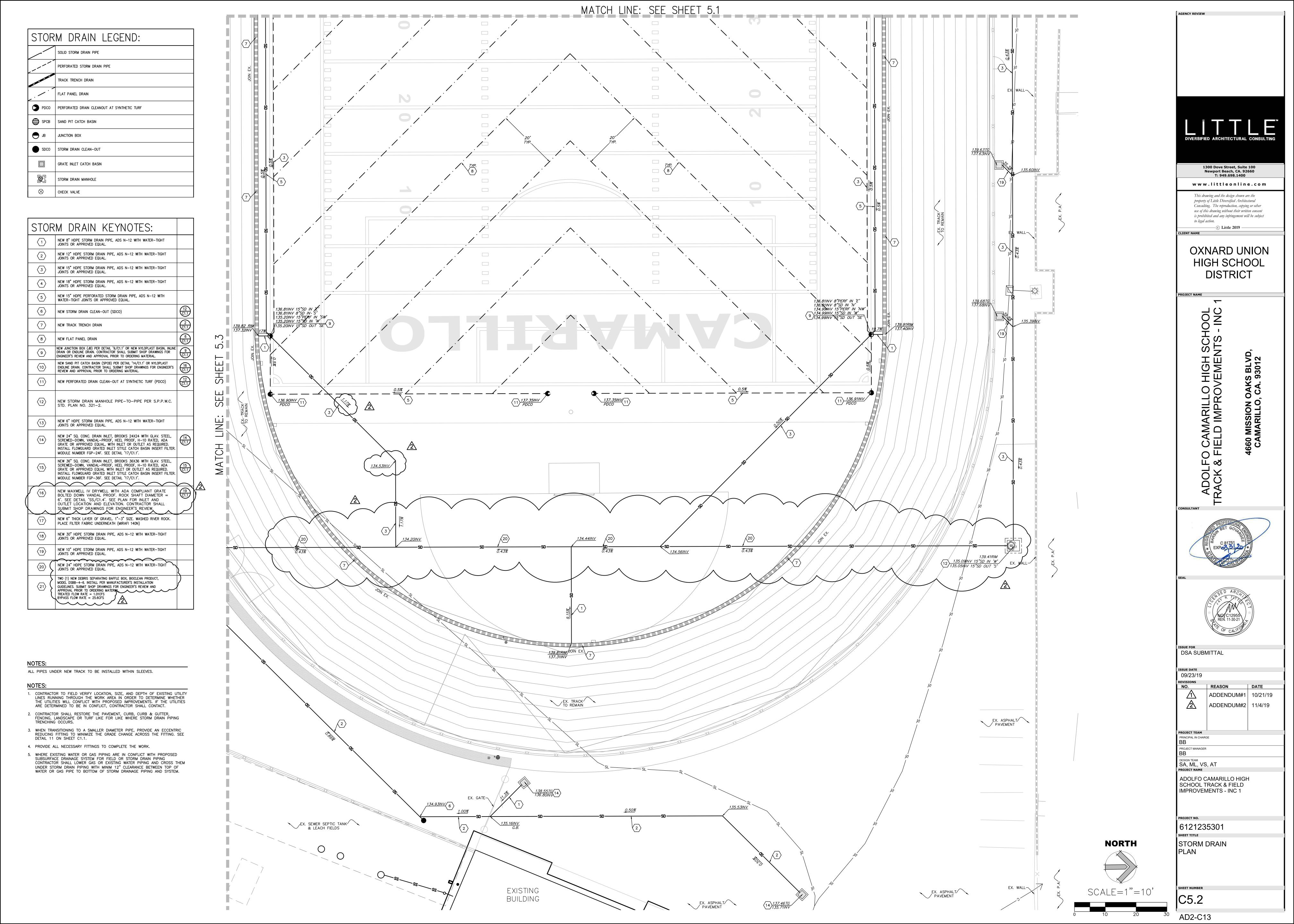
APP. 03-120008

ADDENDUM #2

REF. DWG.: C5.1

ISSUE DATE: 11/14/19

AD2-C12

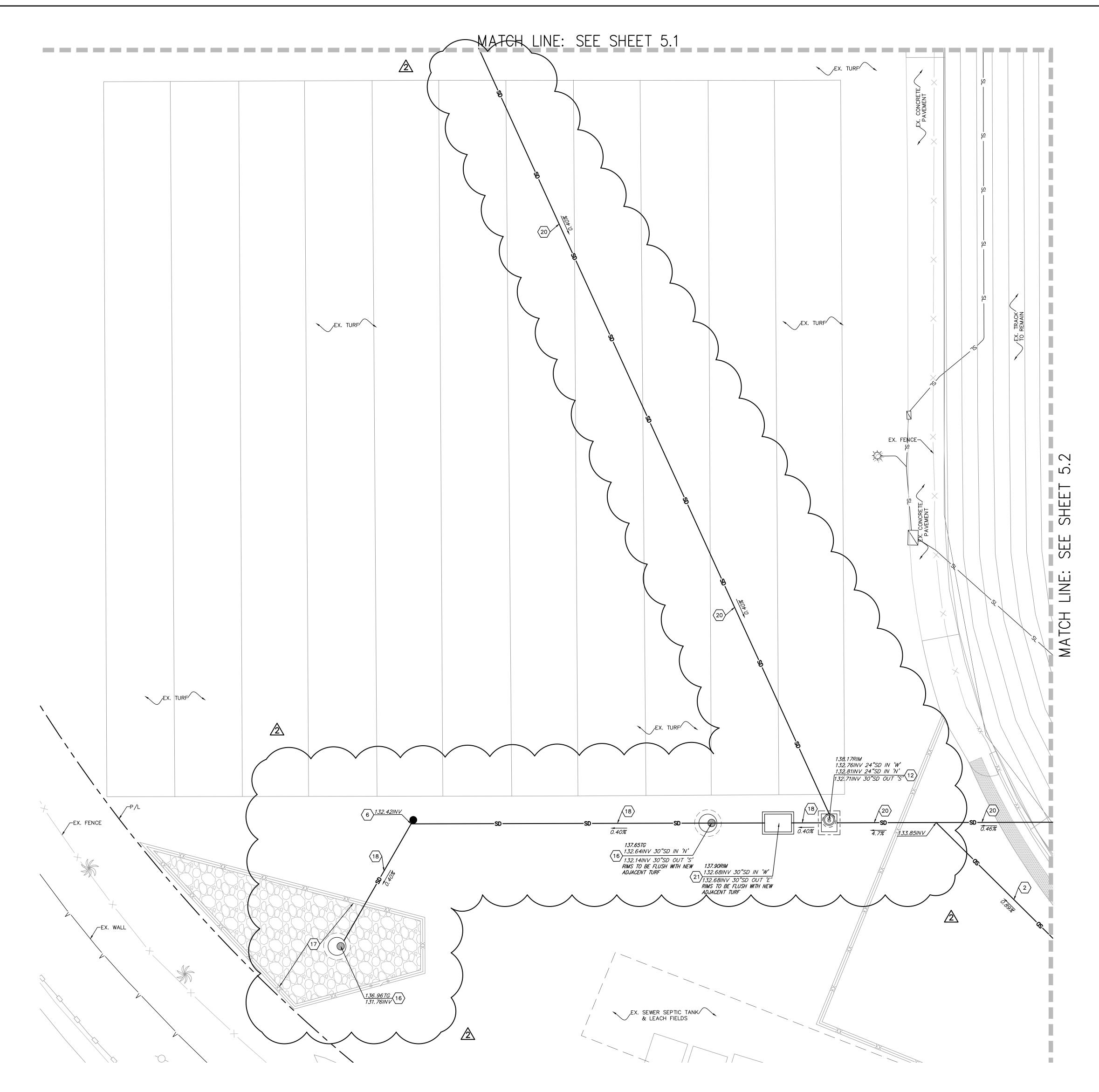


S	TOR	M DRAIN KEYNOTES:	
	1	NEW 8" HDPE STORM DRAIN PIPE, ADS N-12 WITH WATER-TIGHT JOINTS OR APPROVED EQUAL.	
	2	NEW 12" HDPE STORM DRAIN PIPE, ADS N-12 WITH WATER-TIGHT JOINTS OR APPROVED EQUAL.	
	3	NEW 15" HDPE STORM DRAIN PIPE, ADS N-12 WITH WATER-TIGHT JOINTS OR APPROVED EQUAL.	
	4	NEW 18" HDPE STORM DRAIN PIPE, ADS N-12 WITH WATER-TIGHT JOINTS OR APPROVED EQUAL.	
	5	NEW 15" HDPE PERFORATED STORM DRAIN PIPE, ADS N-12 WITH WATER-TIGHT JOINTS OR APPROVED EQUAL.	
	6	NEW STORM DRAIN CLEAN-OUT (SDCO)	12 C1.1
	7	NEW TRACK TRENCH DRAIN	(2) (C1.1)
	8	NEW FLAT PANEL DRAIN	6 C1.1
	9	NEW JUNCTION BOX (JB) PER DETAIL '9/C1.1' OR NEW NYLOPLAST BASIN, INLINE DRAIN OR ENDLINE DRAIN. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR ENGINEER'S REVIEW AND APPROVAL PRIOR TO ORDERING MATERIAL.	9 C1.1
	(10)	NEW SAND PIT CATCH BASIN (SPCB) PER DETAIL '14/C1.1' OR NYLOPLAST ENDLINE DRAIN. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR ENGINEER'S REVIEW AND APPROVAL PRIOR TO ORDERING MATERIAL.	16 C1.1
	(11)	NEW PERFORATED DRAIN CLEAN-OUT AT SYNTHETIC TURF (PDCO)	12 C1.1
	(12)	NEW STORM DRAIN MANHOLE PIPE—TO—PIPE PER S.P.P.W.C. STD. PLAN NO. 321—2.	
	(13)	NEW 6" HDPE STORM DRAIN PIPE, ADS N-12 WITH WATER-TIGHT JOINTS OR APPROVED EQUAL.	
	(14)	NEW 24" SQ. CONC. DRAIN INLET, BROOKS 24X24 WITH GLAV. STEEL, SCREWED-DOWN, VANDAL-PROOF, HEEL PROOF, H-10 RATED, ADA GRATE OR APPROVED EQUAL, WITH INLET OR OUTLET AS REQUIRED. INSTALL FLOWGUARD GRATED INLET STYLE CATCH BASIN INSERT FILTER. MODULE NUMBER FGP-24F. SEE DETAIL '17/C1.1'.	14 C1.1
	(15)	NEW 36" SQ. CONC. DRAIN INLET, BROOKS 36X36 WITH GLAV. STEEL, SCREWED-DOWN, VANDAL-PROOF, HEEL PROOF, H-10 RATED, ADA GRATE OR APPROVED EQUAL WITH INLET OR OUTLET AS REQUIRED. INSTALL FLOWGUARD GRATED INLET STYLE CATCH BASIN INSERT FILTER. MODULE NUMBER FGP-36F. SEE DETAIL '17/C1.1'.	(15) (C1.1)
	(16)	NEW MAXWELL IV DRYWELL WITH ADA COMPLIANT GRATE BOLTED DOWN VANDAL PROOF. ROCK SHAFT DIAMETER = 6'. SEE DETAIL 'SS/C1.4'. SEE PLAN FOR INLET AND OUTLET LOCATION AND ELEVATION. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR ENGINEER'S REVIEW.	19 (1.1)
_	(17)	NEW 6" THICK LAYER OF GRAVEL. 1"-3" SIZE. WASHED RIVER ROCK. PLACE FILTER FABRIC UNDERNEATH (MIRAFI 140N)	
	(18)	NEW 30" HDPE STORM DRAIN PIPE, ADS N-12 WITH WATER-TIGHT JOINTS OR APPROVED EQUAL.	
	(19)	NEW 10" HDPE STORM DRAIN PIPE, ADS N-12 WITH WATER-TIGHT JOINTS OR APPROVED EQUAL.	
	\bigcirc 20 \bigcirc	NEW 24" HDPE STORM DRAIN PIPE, ADS N-12 WITH WATER-TIGHT JOINTS OR APPROVED EQUAL.	\
	(21)	TWO (1) NEW DEBRIS SEPARATING BAFFLE BOX, BIOCLEAN PRODUCT, MODEL DSBB-4-6. INSTALL PER MANUFACTURER'S INSTALLATION GUIDELINES. SUBMIT SHOP DRAWINGS FOR ENGINEER'S REVIEW AND APPROVAL PRIOR TO ORDERING MATERIAL. TREATED FLOW RATE = 1.01CFS BYPASS FLOW RATE = 25.6CFS)

STOR	M DRAIN LEGEND:
	SOLID STORM DRAIN PIPE
	PERFORATED STORM DRAIN PIPE
	TRACK TRENCH DRAIN
. / .	FLAT PANEL DRAIN
PDC0	PERFORATED DRAIN CLEANOUT AT SYNTHETIC TURF
⊜ SPCB	SAND PIT CATCH BASIN
● JB	JUNCTION BOX
SDCO	STORM DRAIN CLEAN-OUT
	GRATE INLET CATCH BASIN
	STORM DRAIN MANHOLE
\otimes	CHECK VALVE

ALL PIPES UNDER NEW TRACK TO BE INSTALLED WITHIN SLEEVES.

- 1. CONTRACTOR TO FIELD VERIFY LOCATION, SIZE, AND DEPTH OF EXISTING UTILITY LINES RUNNING THROUGH THE WORK AREA IN ORDER TO DETERMINE WHETHER THE UTILITIES WILL CONFLICT WITH PROPOSED IMPROVEMENTS. IF THE UTILITIES ARE DETERMINED TO BE IN CONFLICT, CONTRACTOR SHALL CONTACT.
- CONTRACTOR SHALL RESTORE THE PAVEMENT, CURB, CURB & GUTTER, FENCING, LANDSCAPE OR TURF LIKE FOR LIKE WHERE STORM DRAIN PIPING TRENCHING OCCURS.
- WHEN TRANSITIONING TO A SMALLER DIAMETER PIPE, PROVIDE AN ECCENTRIC REDUCING FITTING TO MINIMIZE THE GRADE CHANGE ACROSS THE FITTING. SEE DETAIL 11 ON SHEET C1.1.
- 4. PROVIDE ALL NECESSARY FITTINGS TO COMPLETE THE WORK.
- 5. WHERE EXISTING WATER OR GAS PIPING ARE IN CONFLICT WITH PROPOSED SUBSURFACE DRAINAGE SYSTEM FOR FIELD OR STORM DRAIN PIPING CONTRACTOR SHALL LOWER GAS OR EXISTING WATER PIPING AND CROSS THEM UNDER STORM DRAIN PIPING WITH MINIM 12" CLEARANCE BETWEEN TOP OF WATER OR GAS PIPE TO BOTTOM OF STORM DRAINAGE PIPING AND SYSTEM.





1300 Dove Street, Suite 100 Newport Beach, CA. 92660 T: 949.698.1400

AGENCY REVIEW

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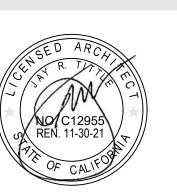
OXNARD UNION HIGH SCHOOL

DISTRICT

VEMENTS - INC O CAMARILLO P FIELD IMPROV

4660 MISSION OAK CAMARILLO, CA.





DSA SUBMITTAL

09/23/19

REASON ADDENDUM#1 10/21/19 ADDENDUM#2 11/4/19

PROJECT TEAM PRINCIPAL IN CHARGE PROJECT MANAGER

DESIGN TEAM
SA, ML, VS, AT
PROJECT NAME

ADOLFO CAMARILLO HIGH SCHOOL TRACK & FIELD IMPROVEMENTS - INC 1

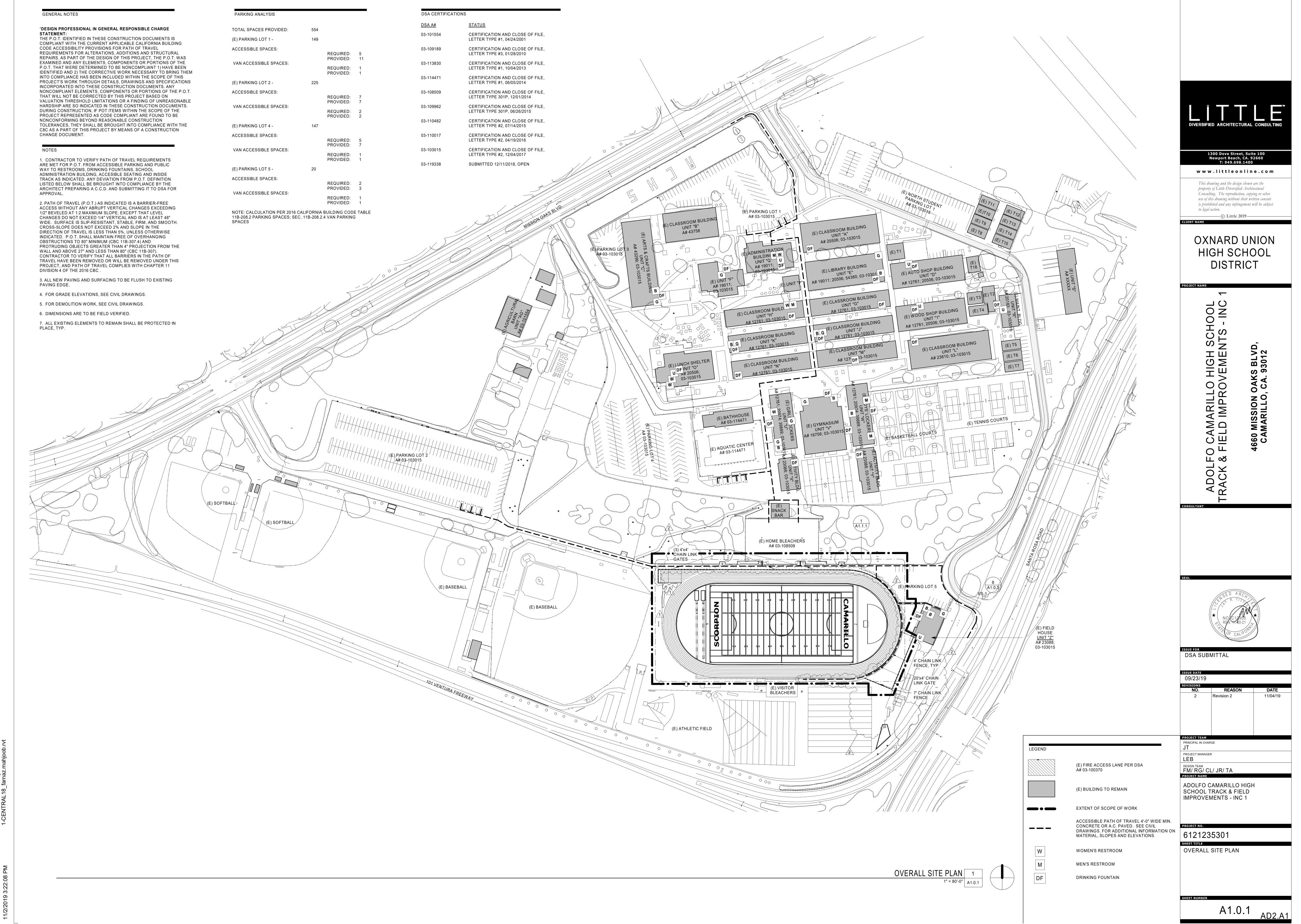
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STORM DRAIN PLAN

NORTH

SCALE=1"=10'

AD2-C14



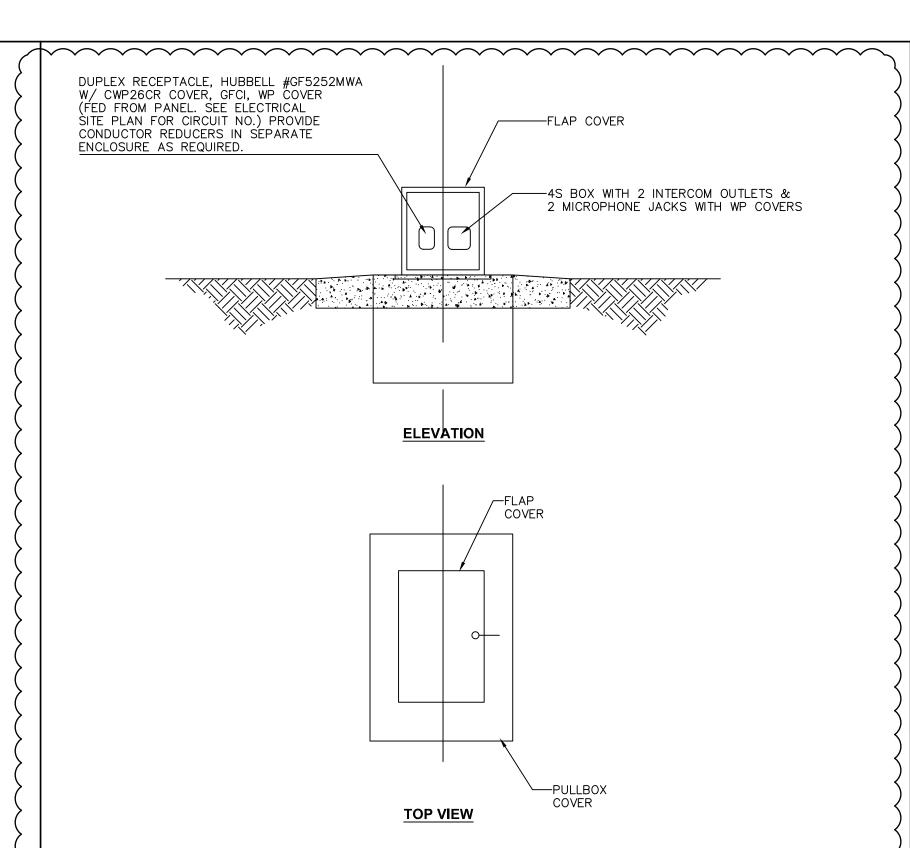


- 1. All work is to be performed per the 2016 issue of the California Electrical Code and the 2016 California Energy Code as accepted by the City of DXNARD and all other applicable national, state and local codes and laws pertaining to electrical
- 2. All work in hazardous locations shall comply with CEC Art. 500 through 516 as applicable.
- 3. Nothing in these notes shall be construed as circumventing any more stringent specification or requirement of the contract documents.
- 4. Electrical Contractor shall visit the job site prior to bidding work and include in his bid the necessary costs required to complete this project according to the intent of the drawings.
- 5. Any discrepancies between site conditions and drawings shall be brought to the
- attention of the project coordinator or Architect prior to bid if possible. 6. Electrical work under this contract shall include all labor, materials and equipment necessary to complete the installation covered under the contract including control conduit and wiring as documented or inferred in the mechanical drawings.
- 7. All material and equipment furnished and or installed under this contract shall be new, free from defects, and shall be guaranteed for a period of one year from the date of final acceptance by owner or his representative. Should any problems develop during this warranty period due to faulty workmanship, material defects or equipment defects or failure, the Electrical Contractor shall correct the problem and repair or replace equipment or material without cost to the owners. All work shall be executed in a orkmanlike manner and shall be neat in appearance as well as functional when completed.
- 8. Unless noted otherwise or coordinated with the General Contractor, the Electrical Contractor shall be responsible for all
- demolition, cutting, and patching relating to electrical work.
- 9. State handicap requirements are to be met per standards listed in "SYMBOL LIST". 10. Cut sheets shall be provided by Electrical Contractor for all equipment provided within contract scope of work.

MATERIAL and INSTALLATION

- 1. All electrical materials and equipment are to be Underwriter's laboratory listed or listed by an equivalent nationally recognized testing laboratory accepted by the City of DXNARD. All materials shall be approved for the intended purpose and used for such purpose
- 2. All 600-volt insulated wire in conduits shall be copper type THHN/THWN-2 unless
- 3. All conductors size AWG #12 and smaller shall be solid, all conductors size #10 and larger shall be stranded.
- 4. All junction boxes shall be marked (in ink) with the panel number, circuit numbers, and system voltage contain within, ("Magic Markers" are acceptable). i.e. 'LA'-1,3,5 277/480V or 'RA'-2,4,6 120/208V etc.
- 5. When conduit must cross traffic areas, the conduit shall cross perpendicular to the normal traffic pattern.
- 6. All ballasts are to be CEC listed.
- 7. All outdoor lighting fixtures are to be listed for wet or damp location depending on type of exposure.
- 8. All devices shall be grounded by means of a separate grounding conductor and
- either a wire bond from the device strap to the box or a self-grounding screw. 9. Each multiwire branch circuit shall be provided with a means that will simultaneously disconnect all ungrounded conductors at the point where the branch circuit
- originates. (CEC 210.4(B)) 10. The ungrounded and grounded conductors of each multiwire branch circuit shall be grouped by wire ties or similar means in at least one location within the panelboard
- or other point of origination. (CEC 210.4(D)) 11. All new overcurrent devices installed in existing panels / switchboards shall match or exceed the make, model and interrupting capacity of the existing overcurrent

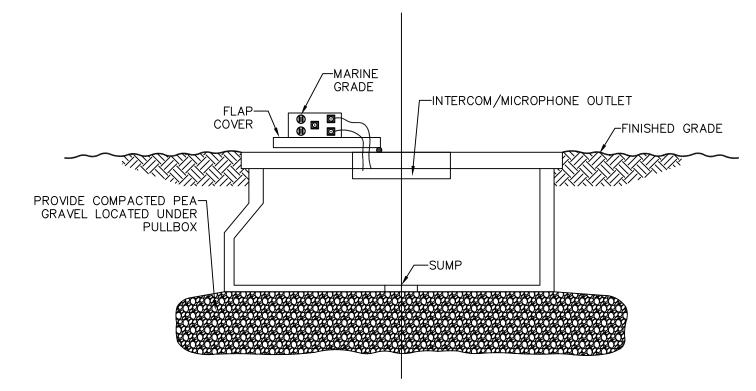
- 1. Upon completion of work, Electrical Contractor shall insure the installation to be free from short circuits, phase grounds and neutral grounds.
- 2. All feeders shall have insulation tested prior to energization. 3. All panels, transformers, distribution boards, switches, etc. shall be labeled per Single Line Diagram using plastic plates with 3/8" high white letters on black backgrounds. Label shall include item name and voltage present. Transformer label shall include both primary and secondary voltages. Label shall be permanently attached using at least (2) round head stainless steel machine screws with minimum thread size 8-32.
- 4. Electrical Contractor shall furnish as-built drawings to Architect upon completion
- 5. Electrical Contractor shall be available for night inspection and approval of
- 6. Prior to final energization, neutral feed shall be disconnected from the panel and bus with all load neutrals connected shall be tested in the presence of the electrical engineer for faults to ground.
- 7. All circuit breaker, neutral and ground lug connections shall be torqued per manufacturer's specifications in the presence of the electrical inspector.
- 8. The issuance of a permit shall not prevent the Building Official from requiring the correction of errors on these plans or from preventing any violation of the codes adopted by the city, relevant laws, ordinances, rules and/or regulations.



GENERAL NOTES

1. THE TRACK SURFACE MATERIAL SHALL COVER PULLBOX COVER AND LID. 2. ALL PULLBOXES NEAR TYRACK SHALL BELOCATED ON EDGE OF TRACK.

\POWER/INTERCOM/MICROPHONE OUTLET TERMINAL DETAIL E-000 NO SCALE



GENERAL NOTES

1. THE TRACK SURFACE MATERIAL SHALL COVER PULLBOX COVER AND LID. 2. ALL PULLBOXES NEAR TRACK SHALL BE LOCATED ON EDGE OF TRACK.

B PULLBOX MOUNTING DETAIL E-000 NO SCALE

INSTALLATION DETAILS | SCALE: NONE





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OXNARD UNION HIGH SCHOOL **DISTRICT**

> 400L - INC ARILLO IMPRO\ MISSION

AM,

ADOL RACK

NO. E-21460



DESIGN DEVELOPMENT

10/21/19 ADDENDUM PRINCIPAL IN CHARGE B.E.S. PROJECT MANAGER ADOLFO CAMARILLO HIGH

IMPROVEMENTS - INC 1

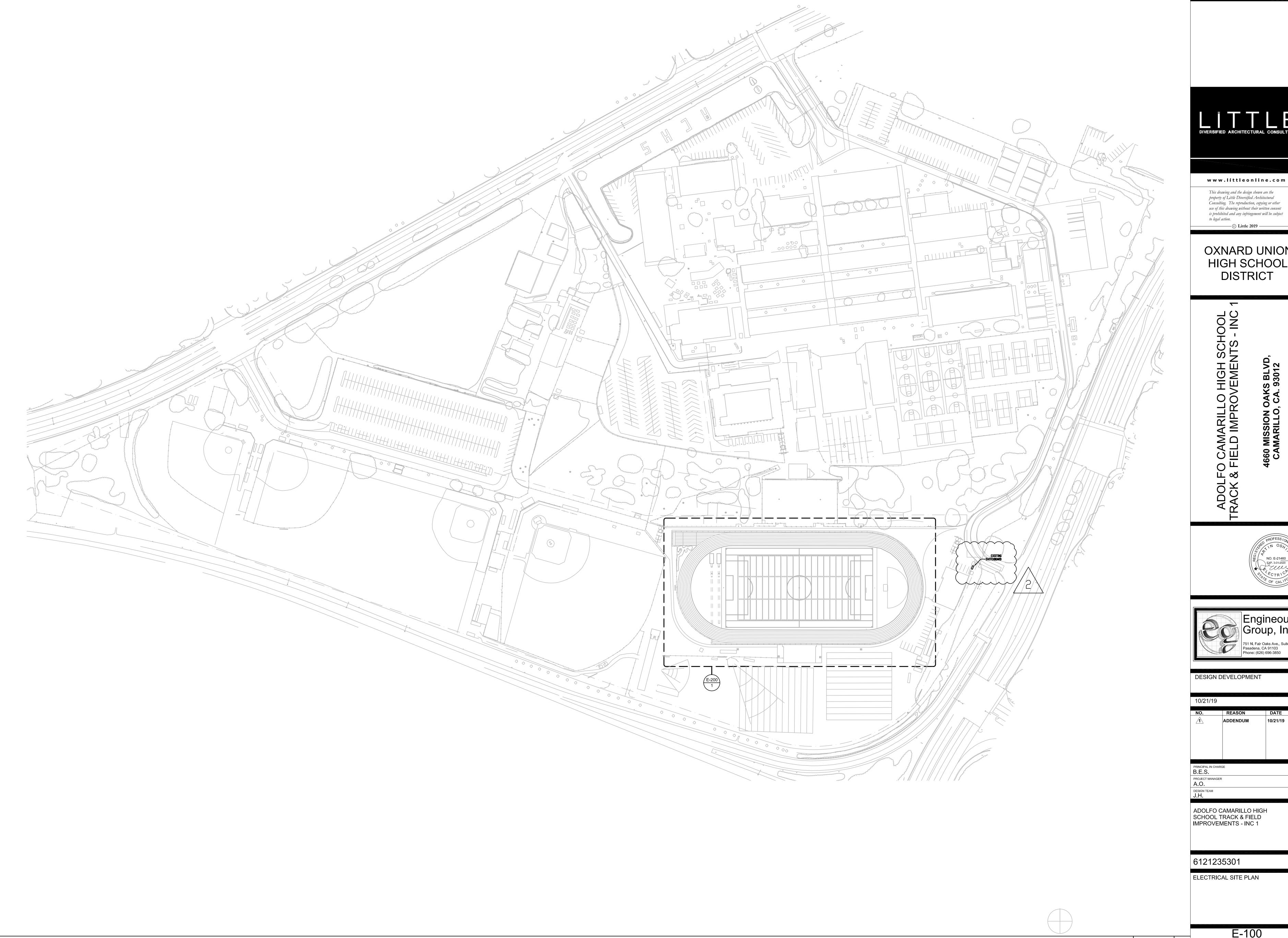
SCHOOL TRACK & FIELD

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SYMBOLS AND NOTES

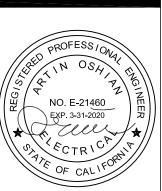
E-000

NOTES | SCALE: NONE

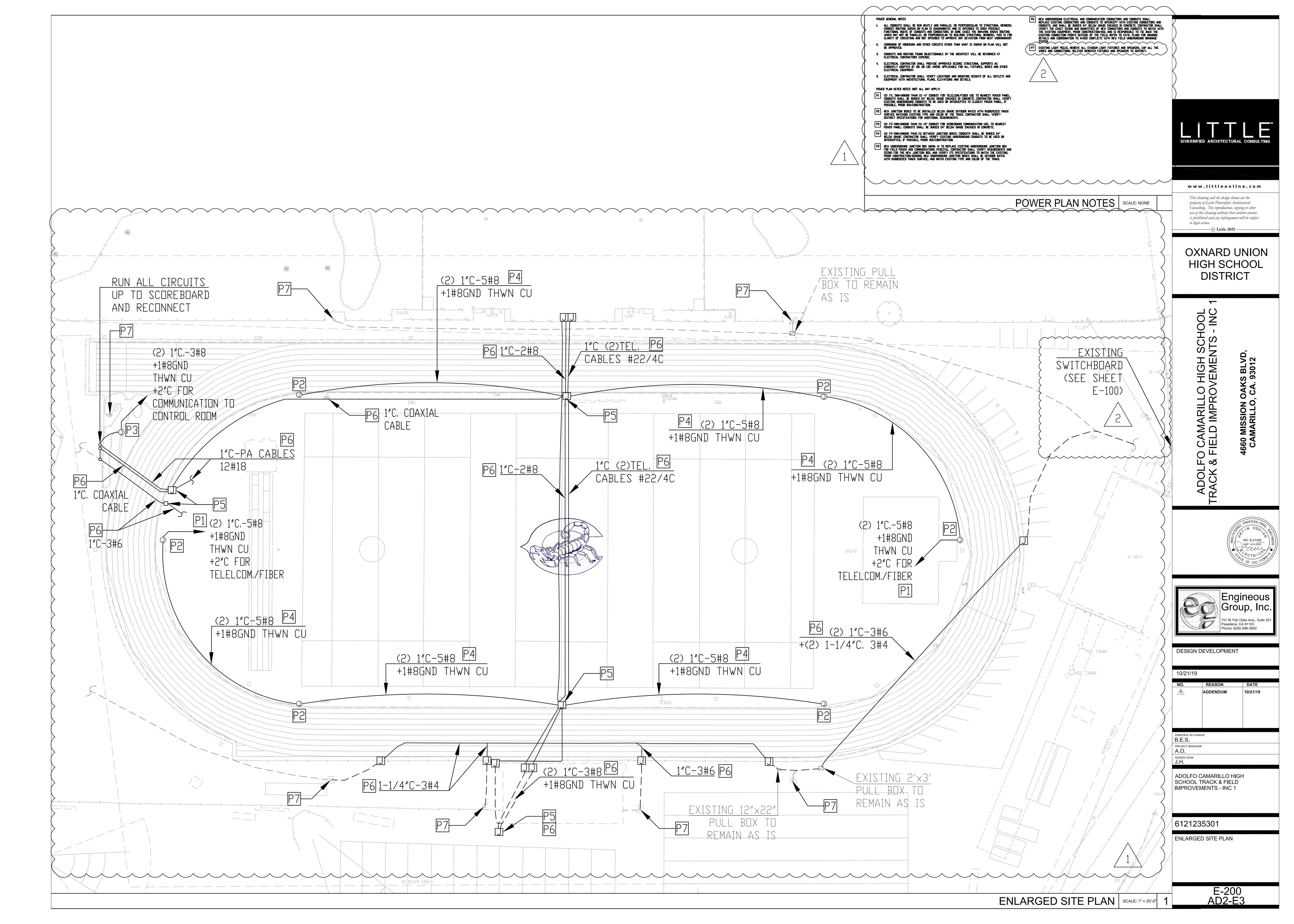


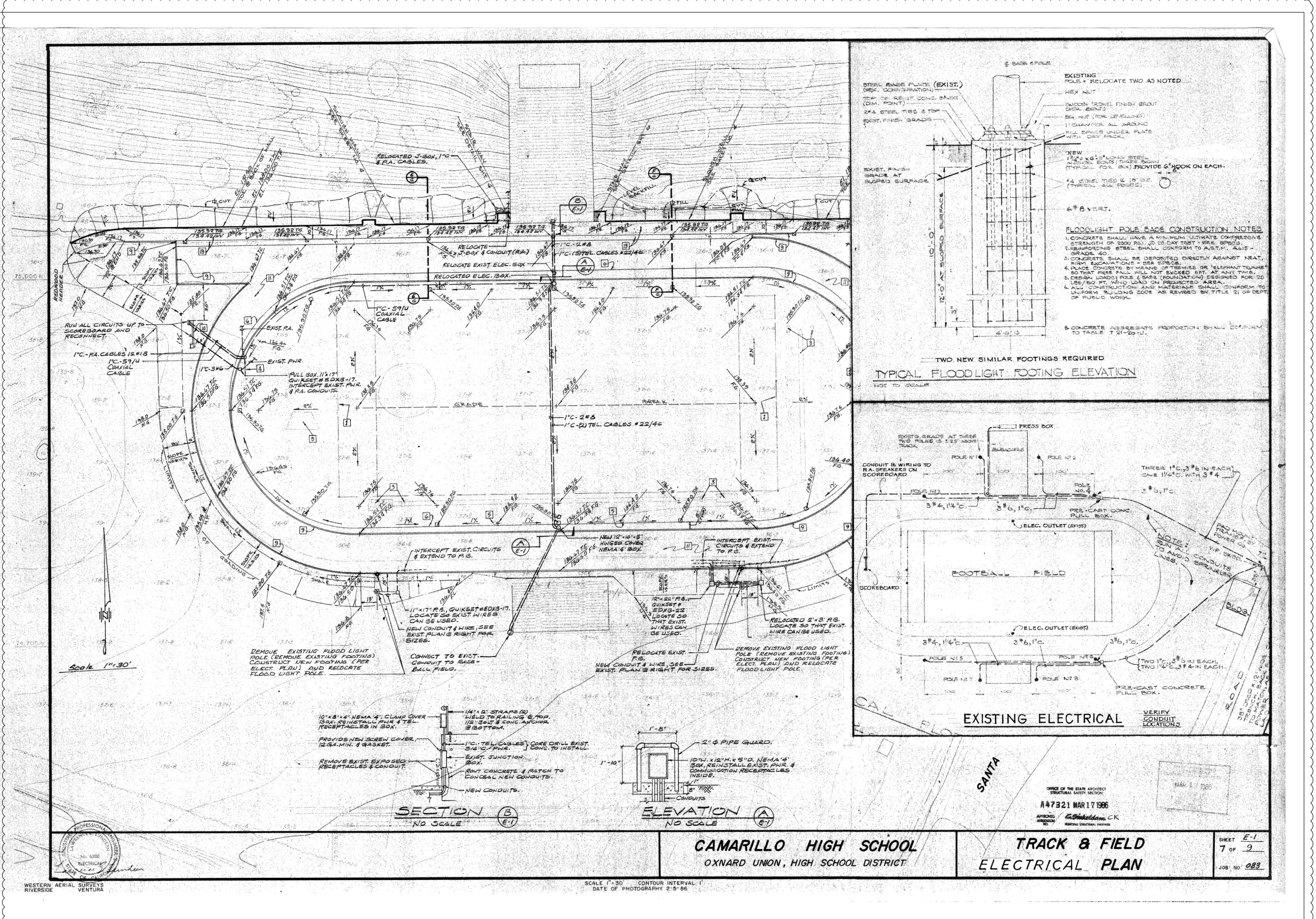
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OXNARD UNION HIGH SCHOOL DISTRICT

HIGH SCHOOL VEMENTS - INC 1

RACK & FIELD IMPROVEME





DESIGN DEVELOPMENT

NO.	REASON	DATE
<u>1</u>	ADDENDUM	10/21/19

B.E.S.

PROJECT MANAGER
A.O.

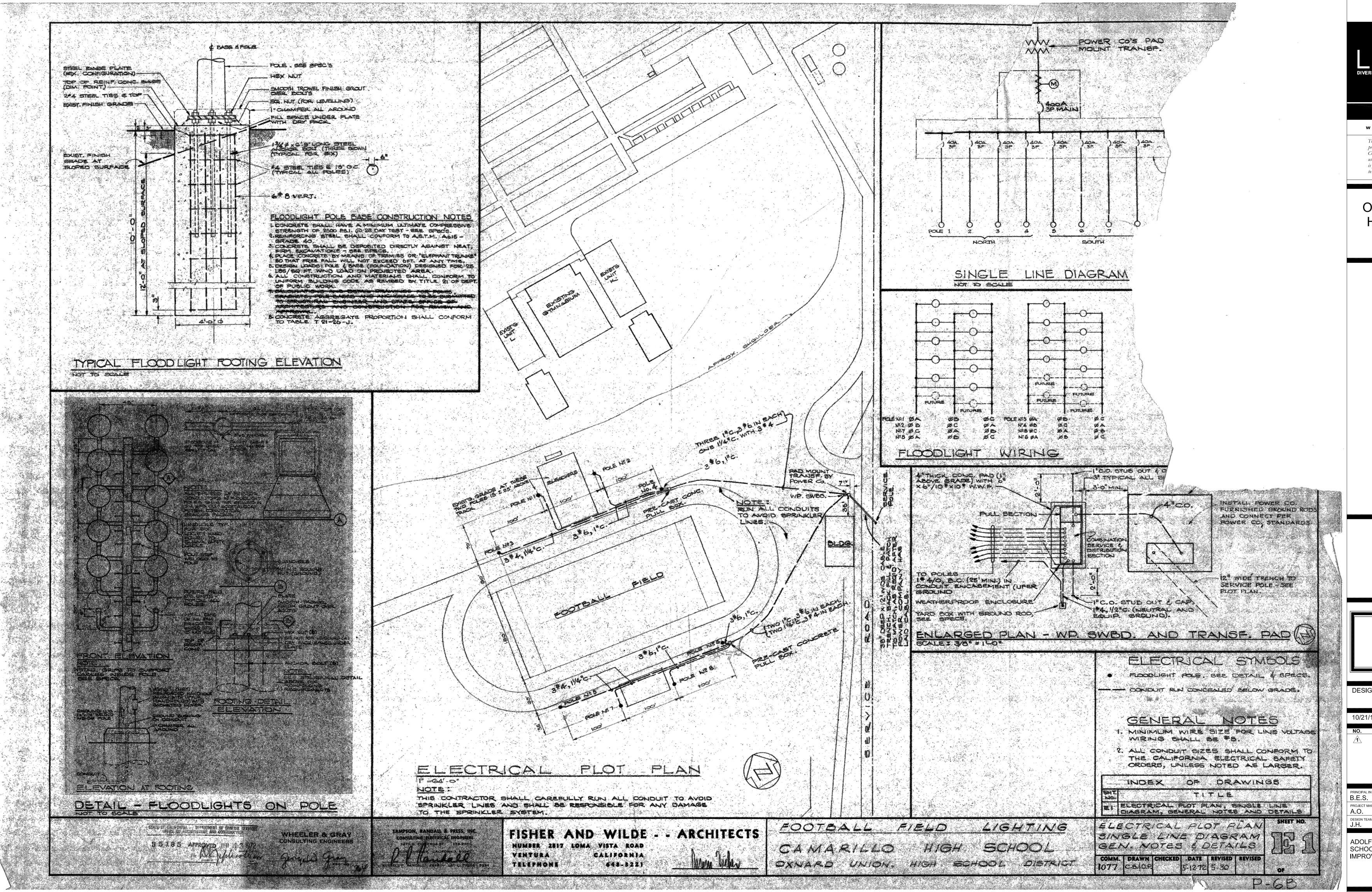
DESIGN TEAM

ADOLFO CAMARILLO HIGH SCHOOL TRACK & FIELD IMPROVEMENTS - INC 1

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ELECTRICAL AS-BUILT PLANS FOR REFERENCE ONLY





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OXNARD UNION HIGH SCHOOL DISTRICT



Engineous Group, Inc.

DESIGN DEVELOPMENT

10/21/19 PRINCIPAL IN CHARGE B.E.S.

ADOLFO CAMARILLO HIGH SCHOOL TRACK & FIELD IMPROVEMENTS - INC 1

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